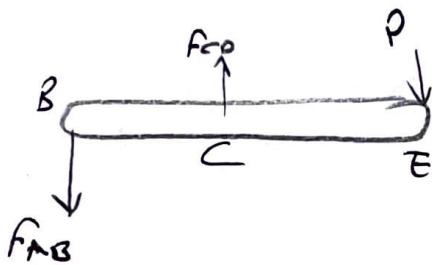


The Answers of Midterm ME 208

1) F.B.D



$$\sum \mathcal{M}_B = 0 \Rightarrow 0,3 F_{CD} - 0,75 P = 0$$

$$\boxed{P = \frac{2}{5} F_{CD}}$$

$$\sum \mathcal{M}_C = 0 \Rightarrow 0,3 F_{AB} - 0,45 P = 0$$

$$\boxed{P = \frac{2}{3} F_{AB}}$$

Note: Both links have the same area, pin diameter and material). Therefore, they have the same ultimate load.

Failure by pin in single shear.

$$A = \frac{\pi}{4} d^2 = \frac{\pi}{4} (0,015)^2 = 490,9 \times 10^{-6} \text{ m}^2$$

$$F_u = 2 \sigma_u A = (210 \times 10^6) (490,9 \times 10^{-6}) = \boxed{103,09 \text{ kN}} \Rightarrow \text{choose smaller } f_u.$$

Failure by tension in link.

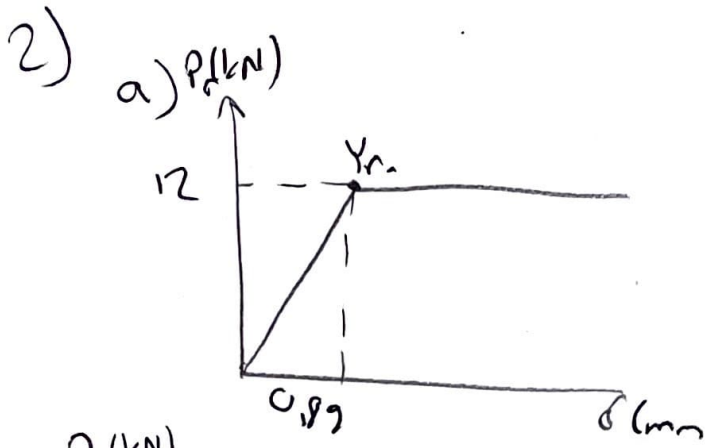
$$A = (b-d)t = (0,05 - 0,015) 0,012 = 3 \times 10^{-4} \text{ m}^2$$

$$F_u = \sigma_u A = (490 \times 10^6) (3 \times 10^{-4}) = 147 \text{ kN}$$

$$F_{all} = \frac{F_u}{F.S.} = \frac{103,09}{3} = 34,36 \text{ kN}$$

Allowable load for structure is the smaller of $\frac{1}{3} F_{all}$ and $\frac{2}{5} F_{all}$

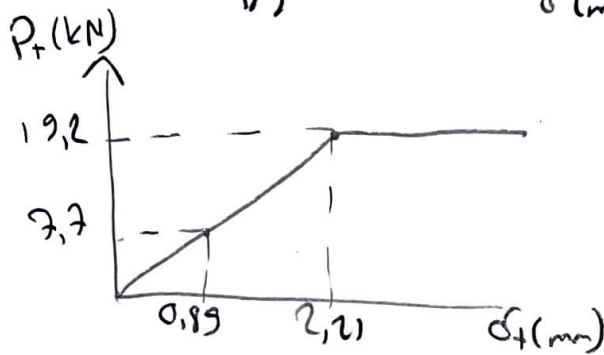
$$P = \frac{2}{5} (34,36) \Rightarrow \boxed{P = 13,7 \text{ kN}}$$



$$P_{Y_r} = \sigma_{Y_r} \cdot A = (250 \times 10^6)(48 \times 10^{-6}) = 12 \text{ kN}$$

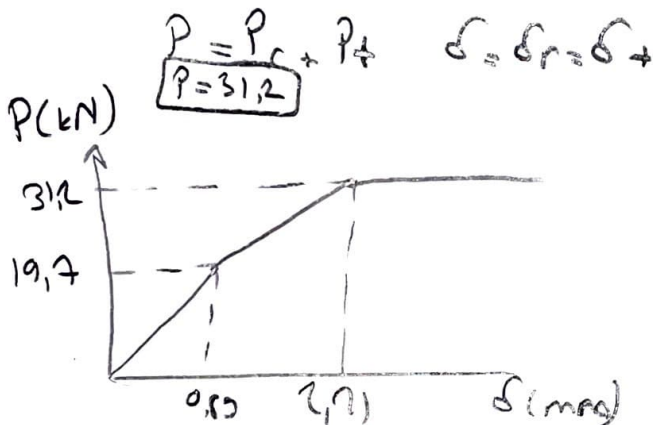
$$\epsilon_{Y_r} = \frac{\sigma_{Y_r}}{E_{Y_r}} = \frac{250 \times 10^6}{210 \times 10^9} = 0.00119$$

$$\delta_{Y_r} = \epsilon_{Y_r} L = 0.00119 \times 0.35 = 0.89 \text{ mm}$$



$$P_{Y_t} = \sigma_{Y_t} \cdot A_t = 310 \times 10^6 + 62 \times 10^6 = 19.2 \text{ kN}$$

$$\delta_{Y_t} = \epsilon_{Y_t} \cdot L = \frac{\sigma_{Y_t}}{E_{Y_t}} \cdot L = \frac{310 \times 10^6}{105 \times 10^9} (0.35 \times 10^3) = 2.21 \text{ mm}$$



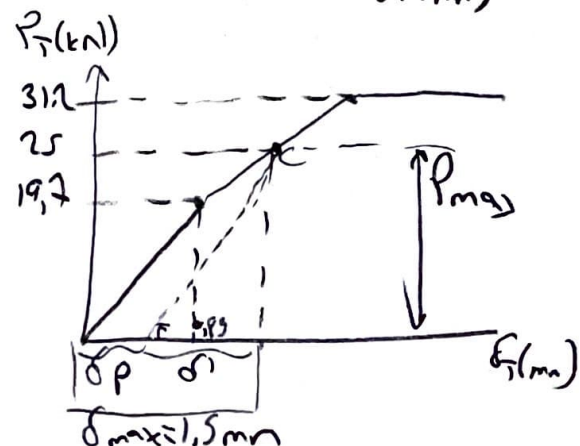
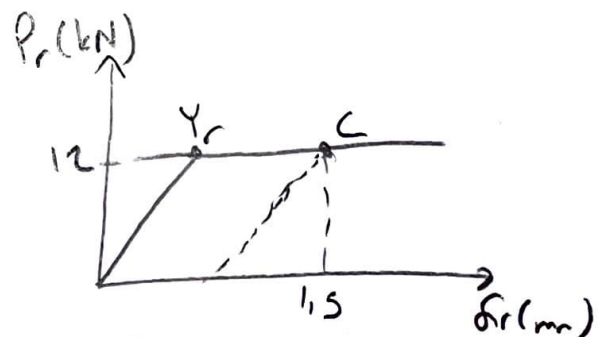
b) $P_r = P_{Y_r} = 12 \text{ kN}$ $P_T = 75 \text{ kN}$

$$P_t = P - P_r = (75 - 12) = 13 \text{ kN}$$

$$\sigma_t = \frac{P_t}{A_t} = \frac{13 \times 10^3}{62 \times 10^{-6}} = 20 \text{ MPa}$$

$$\delta_t = \epsilon_t L = \frac{\sigma_t}{E} L = \frac{210 \times 10^6}{105 \times 10^9} (0.35) = 1.5 \text{ mm}$$

$$\delta_{\max} = \delta_t = 1.5 \text{ mm}$$



c) slope of linear line $\Rightarrow \frac{19.2}{0.89} = 21.1 \text{ kN/mm}$

$$MOY = MCE \Rightarrow \frac{25}{\delta_t} = 21.1 \Rightarrow \delta_t' = 1.131 \text{ mm}$$

$$\delta_p = \delta_{\max} - \delta_t' = 1.5 - 1.131 = 0.369 \text{ mm}$$

3) Solid rod BC: $\tau = \frac{Tc}{J}$ $J = \frac{\pi}{2} c^4$

$\tau_{all} = 25 \times 10^6 \text{ Pa}$ $c = \frac{1}{2} d = 0,015 \text{ m}$

$T_{all} = \frac{\pi}{2} c^3 \tau_{all} = \frac{\pi}{2} (0,015)^3 (25 \times 10^6) = 132,536 \text{ N.m}$

Hollow rod AB: $\tau_{all} = 50 \times 10^6 \text{ Pa}$ $T_{all} = 132,536$

$c_2 = \frac{1}{2} d_2 = \frac{1}{2} (0,075) = 0,0175 \text{ m}$ $c_1 = ?$

$T_{all} = \frac{J \tau_{all}}{c_2} = \frac{\pi}{2} (c_2^4 - c_1^4) \frac{\tau_{all}}{c_2}$

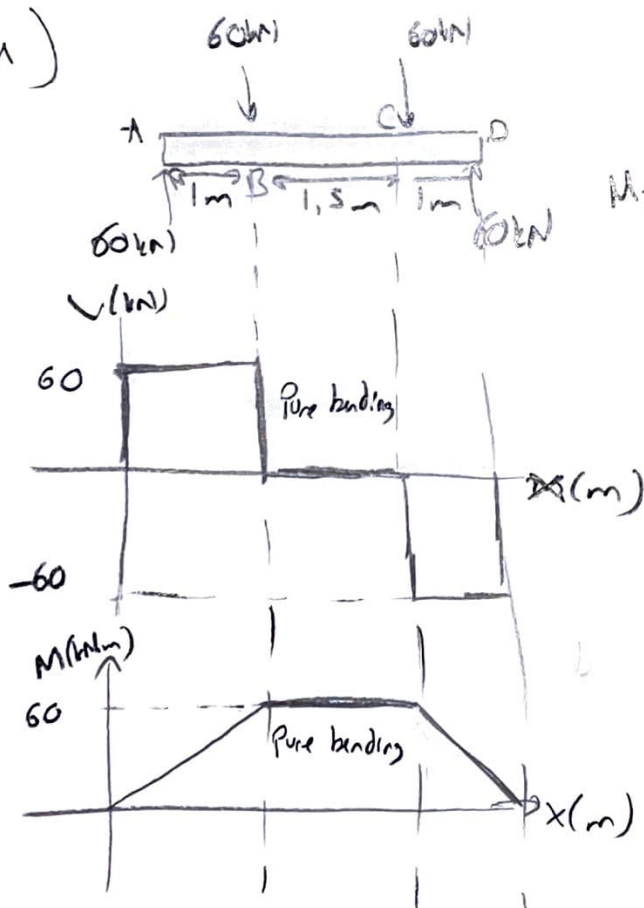
$c_1^4 = c_2^4 - \frac{2 T_{all} c_2}{\pi \tau_{all}} \Rightarrow c_1^4 = 0,0175^4 - \frac{2(132,536)(0,0175)}{\pi (50 \times 10^6)}$

$= 3,3203 \times 10^{-9} \text{ m}^4$

$c_1 = 7,52 \times 10^{-3} \text{ m} = 7,52 \text{ mm}$

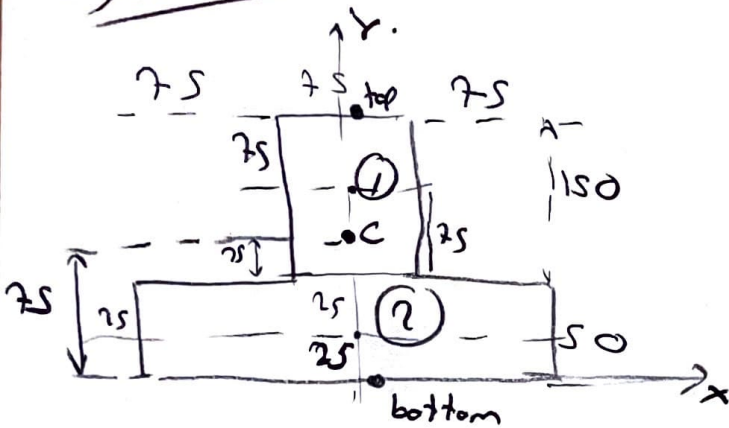
$d_1 = 2c_1 = 15,18 \text{ mm}$

4)



$\Rightarrow$ bending moment = 60 kNm

u) Continued



	A	$\bar{y}_0$	$A \cdot \bar{y}_0$
①	11250	75	106750
②	1250	25	31250
Σ	12500		137500

$$\bar{y}_0 = \frac{137500}{12500} = 11 \text{ mm}$$

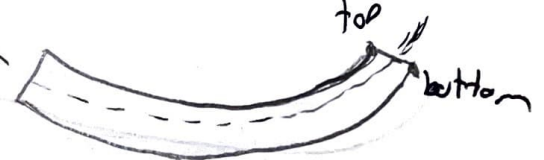
$$I_1 = \frac{1}{12} b_1 h_1^3 + A_1 d_1^2 = \frac{1}{12} (0,075)(0,15)^3 + (0,01125)(0,05)^2 = 49,27 \times 10^{-6} \text{ m}^4$$

$$I_2 = \frac{1}{12} b_2 h_2^3 + A_2 d_2^2 = \frac{1}{12} (0,225)(0,05)^3 + (0,01175)(0,05)^2 = 30,45 \times 10^{-6} \text{ m}^4$$

$$I = I_1 + I_2 = 79,7 \times 10^{-6} \text{ m}^4$$

tensile stress occurs at the bottom
compressive stress occurs at the top

due to bending M



$$\sigma_{top} = -\frac{Mc}{I} = \frac{(60000)(0,125)}{79,7 \times 10^{-6}} = -94,1 \text{ MPa (compression)}$$

$$\sigma_{bot} = \frac{Mc}{I} = \frac{(60000)(0,075)}{79,7 \times 10^{-6}} = 56,5 \text{ MPa (tension)}$$